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There is a notable discrepancy between the plane strain solution and the proposed solution especially in the range below the fundamental vertical frequency of the surface stratum, $\omega_p=\pi v_p/(2H)$, which is assumed to be 3.0 ω_g in the case shown in Fig. 3 because $\omega_p=\pi v_p/(2H)=(v_p/v_s)\omega_g=(1/0.33)\omega_g=3.0\omega_g$, ω_g is the fundamental horizontal frequency of the surface stratum defined by $\pi v_s/(2H)$. In the range above the fundamental vertical frequency s_{w1} is of the same order as the value by the plane strain solution and s_{w2} is smaller than the value by plane strain solution. The differences both in the region below and above the fundamental vertical frequency ω_p are due to the dynamic response of the surface soil stratum. Below ω_p, the stiffness s_{w1} strongly depends on the frequency, and the damping s_{w2} is very low. The damping in this range is mostly caused by material damping and the radiation damping is absent. Above ω_p, s_{w2} rapidly increases linearly with frequency while s_{w1} is almost constant because a horizontally progressive wave only appears above this frequency.

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To Be Continued on No. 10

正　誤　表（9月号）

頁	段	行	種　別	正	誤
607		↑9	表　3	作動	差動
610			図3（右図）	診断書	診断所
619	右	↓21	数　式	$N_{ij}=-\rho\omega(f_{ij}-f_{ij}^*)/2i$	$N_{ij}=-\rho\omega(f_{ij}-f_{ij}^*)/2$
628		↓5	表　1	−23.0	23.0