

## 15. A Short Note on Kanai's Formula.

By Ryutaro TAKAHASI,  
Earthquake Research Institute.

(Read Dec. 20, 1949.—Received May 31, 1950.)

When a building with rigid floors is vibrating with a period  $T$ , we have, concentrating the mass of pillars at the floors,

$$\left. \begin{array}{l} m_n \ddot{y}_n = E_n I_n y''' \\ m_{n-1} \ddot{y}_{n-1} = E_{n-1} I_{n-1} y''' - E_n I_n y''' \\ \dots \\ m_s \ddot{y}_s = E_s I_s y''' - E_{s+1} I_{s+1} y''' \end{array} \right\} \quad (1)$$

where  $\ddot{y}_s$ 's are accelerations, and  $E_s I_s y'''$  the shearing force in the pillar between  $m_s$  and  $m_{s-1}$ , of which the length is  $I_s$ .

Summing up (1), we have

$$\sum_{r=s}^n m_r \ddot{y}_r = E_s I_s y''' \quad (2)$$

Since  $\ddot{y}_r = -4\pi^2 y_r / T^2$ , we can write (2)

$$-y''' = 4\pi^2 \sum_{r=s}^n m_r y_r / E_s I_s T^2 \equiv S \quad (3)$$

which assumes a constant value, say  $S$ , for the pillar between  $m_s$  and  $m_{s-1}$ .

We assume for the same pillar,

$$\left. \begin{array}{ll} l_s y'' = -k y' & \text{at } x = l_s, \\ l_s y'' = h y' & \text{at } x = 0, \\ y = y_s - y_{s-1} & \text{at } x = l_s, \\ y = 0 & \text{at } x = 0, \end{array} \right\} \quad (4)$$

taking the origin temporarily at  $m_{s-1}$ . Then from (3) and (4) we obtain after a simple calculation

$$S = \frac{12(h+k+hk)}{(12+4h+4k+hk)} \cdot \frac{y_s - y_{s-1}}{l_s^3} \quad (5)$$

Eliminating  $S$  from (3) and (5), we get finally

$$E_s I_s = \frac{12 + 4h + 4k + hk}{3(h+k+hk)} \cdot \frac{\pi^2 l_s^3 \sum_{r=s}^n m_r y_r}{T^2(y_s - y_{s-1})}, \dots \quad (6)$$

which is Kanai's formula generalized.<sup>1)</sup>

For the clamped end of pillars, put  $h$  or  $k$  in the formula equal to  $\infty$ , and for the hinged end  $h$  or  $k=0$ .

Values of  $\frac{12+4h+4k+hk}{3(h+k+hk)}$

		Upper End of Pillar		
		Clamped	Elastic	Hinged
Lower End of Pillar.	Clamped	1/3	(4+k)/3(1+k)	4/3
	Elastic	(4+h)/3(1+h)	$\frac{(12+4h+4k+hk)}{3(h+k+hk)}$	$4(3+h)/3h$
	Hinged	4/3	$4(3+k)/3k$	$\infty$

1) Kiyoshi KANAI, *Monthly Coll. Earthq. Res. Inst., Tokyo Univ.* Nov. 15, 1949.

## 15. 金井式小引

地震研究所 高橋龍太郎

建物の振動実験の結果から各階の柱の剛性を求める金井清氏の式の擴張として

$$E_s I_s = \frac{12 + 4h + 4k + hk}{3(h+k+hk)} \cdot \frac{\pi^2 l_s^3 \sum_{s=s}^n m_r y_r}{T^2(y_s - y_{s-1})}$$

なる式を得た。此處に  $k$  及び  $h$  は夫々柱の上及び下の節點の條件による常数で、剛節の場合  $\infty$  に、鉢節の場合 0 に取るべきものである。他の記號は金井式の場合と同様である。